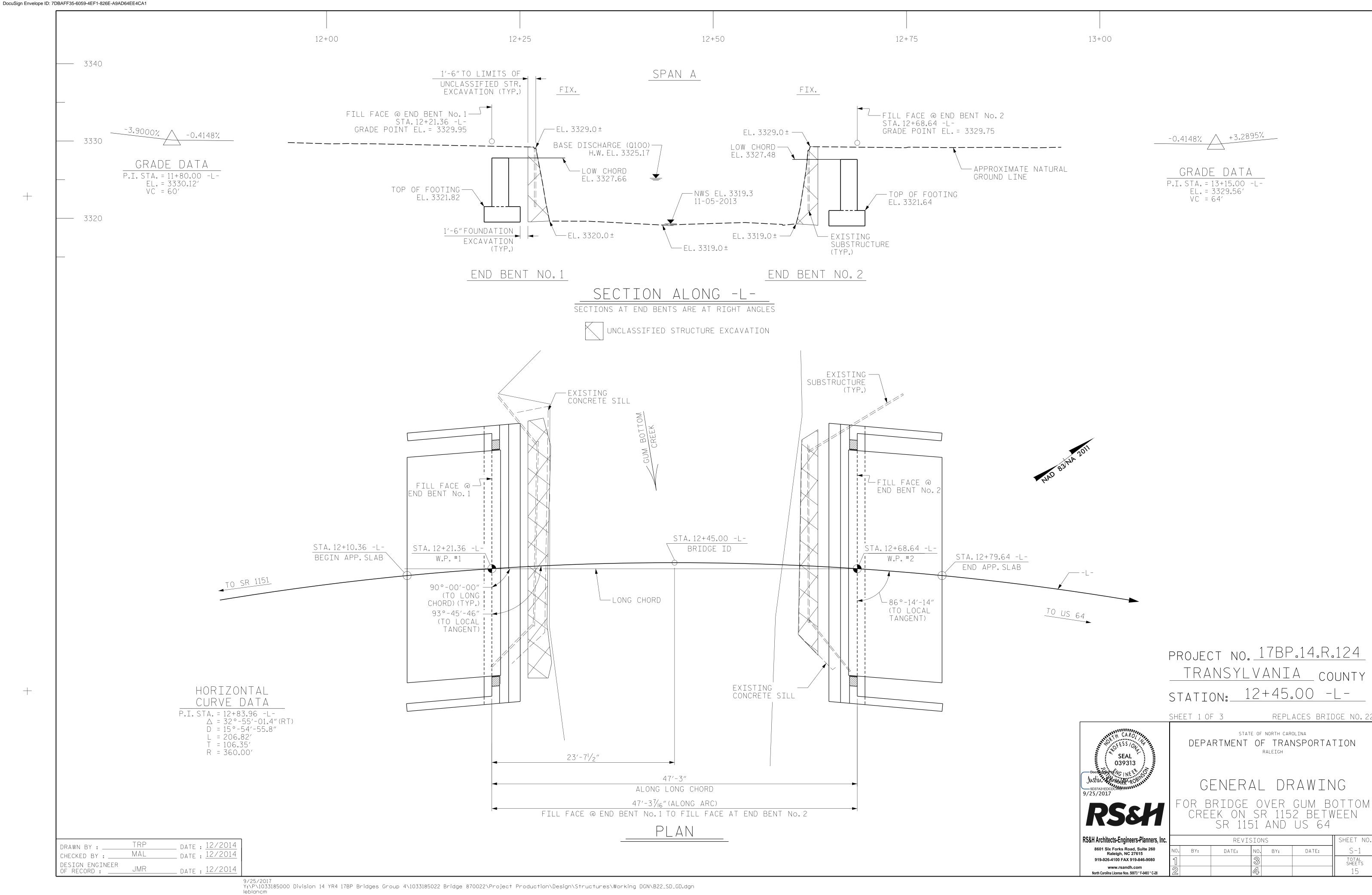
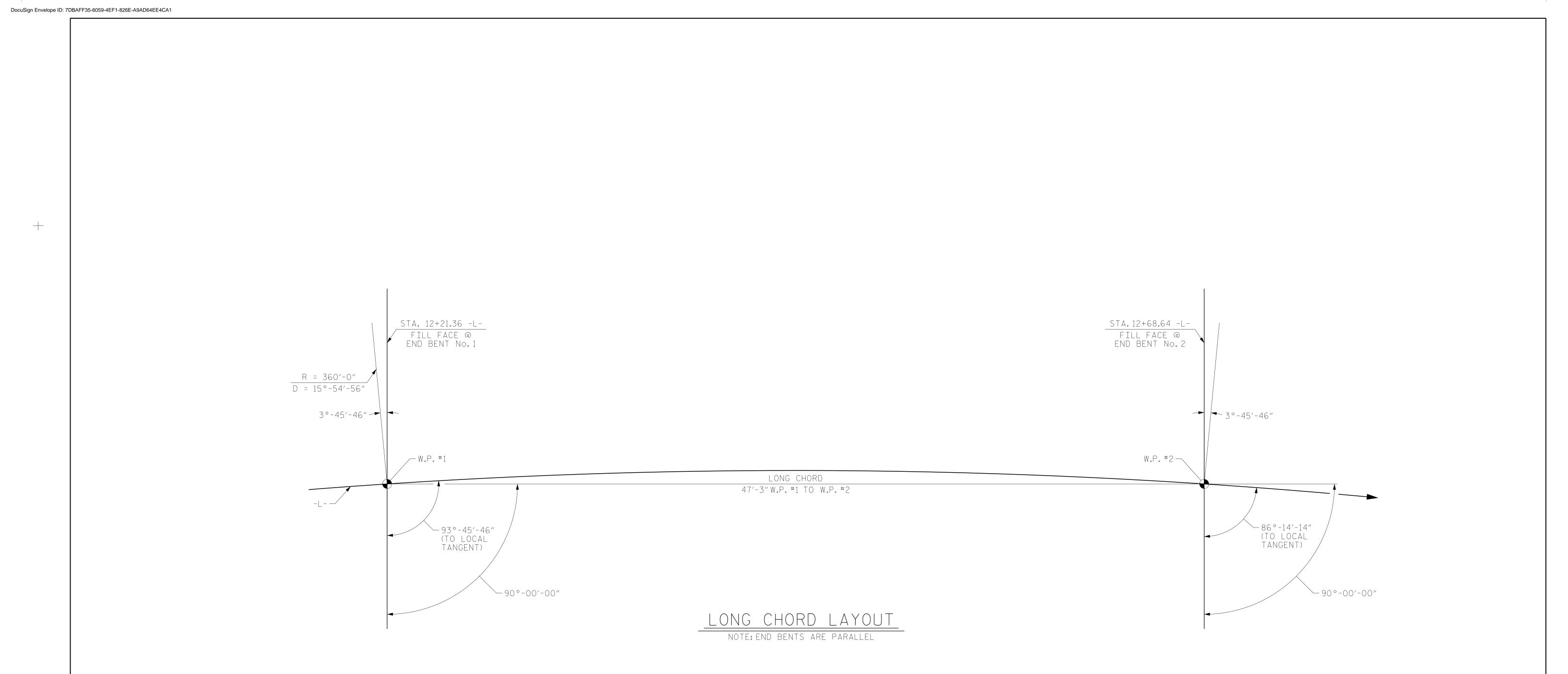
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PROJECT NO. <u>178P.14.R.124</u> TRANSYLVANIA_ COUNTY STATION: 12+45.00 -L-

SHEET 2 OF 3



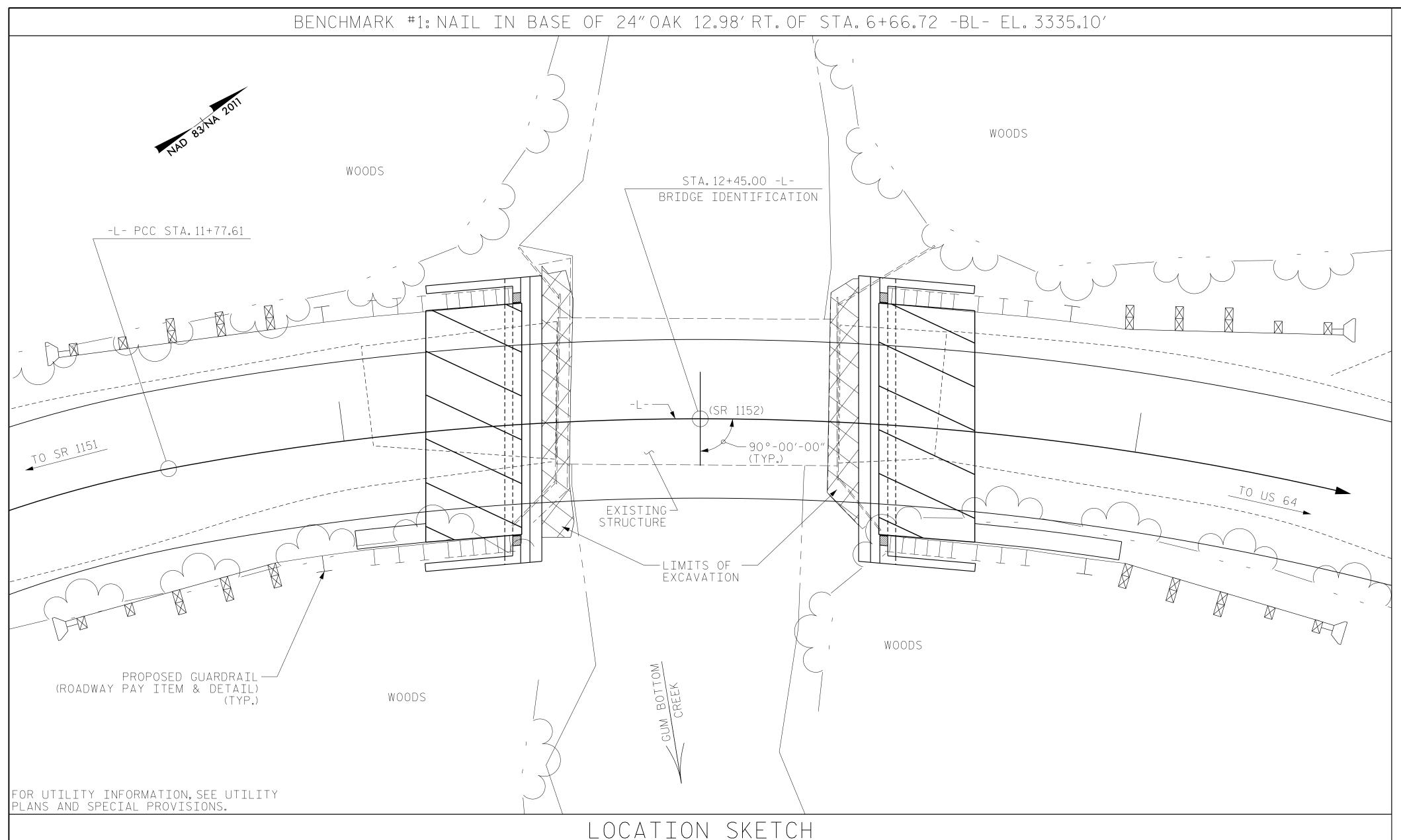
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

RS&H Archit

			O1(110	<i>)</i>	7 (1 1		
hitects-Engineers-Planners, Inc.			REVIS	OIS	VS		SHEET
Six Forks Road, Suite 260 Raleigh, NC 27615	NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
926-4100 FAX 919-846-9080	1			3			TOTAI SHEET
www.rsandh.com				\Box			1 , _

__ DATE : 12/2014 __ DATE : 12/2014 DRAWN BY : _ MAL CHECKED BY : ___ DESIGN ENGINEER of record : ____ __ DATE : <u>12/2014</u>



			TOTAL	BILL	OF M.	ATERIAL	S			
	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION FOR END BENT	UNCLASSIFIED STRUCTURE EXCAVATION	BRIDGE APPROACH SLABS	CLASS A CONCRETE	REINFORCING STEEL	VERTICAL CONCRETE BARRIER RAIL	ELASTOMERIC BEARINGS	3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLABS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LBS.	LIN.FT.	LUMP SUM	NO. LIN.FT.	LUMP SUM
SUPERSTRUCTURE				LUMP SUM			90.00	LUMP SUM	10 450′-0″	LUMP SUM
END BENT 1		LUMP SUM	LUMP SUM		38.5	4429				
END BENT 2		LUMP SUM	LUMP SUM		38.5	4429				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	77.0	8858	90.00	LUMP SUM	10 450'-0"	LUMP SUM

HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION DRAINAGE AREA

BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION

= 3205 CFS = 500 YRS+ = 3329.7

= 1.29 SQ. MI.

= 480 CFS

= 700 CFS

= 3325.17

= 25 YRS

= 3324.5

NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 35'-6"ON A TIMBER DECK ON I-BEAMS AT VARIOUS CENTERS ON TIMBER CAPS AND POST AND SILLS ON CONCRETE FOOTINGS LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS NOT PRESENTLY POSTED FOR A LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 18 FEET LEFT AND 14 FEET RIGHT OF THE CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 12+45.00 -L-."

FOUNDATION NOTES

THE SCOUR CRITICAL ELEVATION FOR END BENTS NO. 1 AND 2 ARE THE BOTTOM OF THE FOOTING ELEVATIONS. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFETIME OF THE STRUCTURE.

THE SPREAD FOOTINGS AT END BENTS NO. 1 AND 2 MAY BE DESIGNED FOR A FACTORED RESISTANCE OF UP TO 10 TSF. CHECK FIELD CONITIONS FOR THE REQUIRED NOMINAL BEARING RESISTANCE OF 22.2 TSF JUST BEFORE PLACING CONCRETE.

KEY IN SPREAD FOOTING AT END BENTS NO.1 AND 2 AT LEAST 12 INCHES INTO WEATHERED ROCK OR AT LEAST 12 INCHES INTO CRYSTALINE ROCK WITH MINIMUM THICKNESS SHOWN ON THE PLANS.

> PROJECT NO. <u>178P.14.R.124</u> TRANSYLVANIA COUNTY STATION: 12+45.00 -L-

SHEET 3 OF 3

SEAL 039313

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North Carolina License Nos. 50073 * F-0493 * C-28

GENERAL DRAWING

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

FOR BRIDGE OVER GUM BOTTOM CREEK ON SR 1152 BETWEEN SR 1151 AND US 64

SHEET NO REVISIONS S-3BY: DATE: DATE: NO. BY: TOTAL SHEETS

_ DATE : <u>12/2014</u> DRAWN BY : . _ DATE : 12/2014 MAL CHECKED BY : _ DESIGN ENGINEER _ DATE : 12/2014 OF RECORD: ___

TRP

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	ENGTH	I LIN	MIT ST	ATE				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM Rating factors (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.088		1.75	0.277	1.34	А	EL	22	0.539	1.23	А	EL	2.2	0.80	0.277	1.09	А	EL	22	
DESIGN		HL-93(0pr)	N/A		1.590		1.35	0.277	1.74	А	EL	22	0.539	1.59	А	EL	2.2	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.336	48.104	1.75	0.277	1.65	А	EL	22	0.539	1.45	А	EL	2.2	0.80	0.277	1.34	А	EL	22	
IVATING		HS-20(0pr)	36.000		1.882	67.763	1.35	0.277	2.14	А	EL	22	0.539	1.88	А	EL	2.2	N/A						
		SNSH	13.500		2.611	35.252	1.4	0.277	4.02	А	EL	22	0.539	4.01	А	EL	2.2	0.80	0.277	2.61	А	EL	22	
		SNGARBS2	20.000		2.108	42.166	1.4	0.277	3.25	А	EL	22	0.539	2.94	А	EL	2.2	0.80	0.277	2.11	А	EL	22	
		SNAGRIS2	22.000		2.067	45.466	1.4	0.277	3.15	А	EL	17.6	0.539	2.77	А	EL	2.2	0.80	0.277	2.07	А	EL	22	
		SNCOTTS3	27.250		1.304	35.527	1.4	0.277	2.01	А	EL	22	0.539	2.01	А	EL	2.2	0.80	0.277	1.30	А	EL	22	
	S	SNAGGRS4	34.925		1.150	40.181	1.4	0.277	1.77	А	EL	22	0.539	1.74	А	EL	2.2	0.80	0.277	1.15	А	EL	22	
		SNS5A	35.550		1.121	39.841	1.4	0.277	1.73	А	EL	22	0.539	1.79	А	EL	2.2	0.80	0.277	1.12	А	EL	22	
		SNS6A	39.950		1.056	42.175	1.4	0.277	1.63	А	EL	22	0.539	1.67	А	EL	2.2	0.80	0.277	1.06	А	EL	22	
LEGAL		SNS7B	42.000	3	1.006	42.268	1.4	0.277	1.55	А	EL	22	0.539	1.68	А	EL	2.2	0.80	0.277	1.01	А	EL	22	
LOAD		TNAGRIT3	33.000		1.296	42.759	1.4	0.277	2	А	EL	22	0.539	1.96	А	EL	2.2	0.80	0.277	1.30	А	EL	22	
RATING		TNT4A	33.075		1.309	43.305	1.4	0.277	2.02	А	EL	22	0.539	1.88	А	EL	2.2	0.80	0.277	1.31	А	EL	22	
		TNT6A	41.600		1.099	45.712	1.4	0.277	1.69	А	EL	22	0.539	1.83	А	EL	2.2	0.80	0.277	1.10	А	EL	22	
		TNT7A	42.000		1.120	47.043	1.4	0.277	1.73	А	EL	22	0.539	1.69	А	EL	2.2	0.80	0.277	1.12	А	EL	22	
		TNT7B	42.000		1.166	48.975	1.4	0.277	1.8	А	EL	22	0.539	1.61	А	EL	2.2	0.80	0.277	1.17	А	EL	22	
		TNAGRIT4	43.000		1.111	47.757	1.4	0.277	1.71	А	EL	22	0.539	1.55	А	EL	2.2	0.80	0.277	1.11	А	EL	22	
		TNAGT5A	45.000		1.033	46.505	1.4	0.277	1.59	А	EL	22	0.539	1.59	А	EL	2.2	0.80	0.277	1.03	А	EL	22	
		TNAGT5B	45,000		1 009	45 408	1 4	0.277	1 56	Δ	FI	22	0.539	1 47	Δ	FI	2.2	0.80	0.277	1 01	Δ	FI	22	

1.009 | 45.408 | 1.4 | 0.277 | 1.56 | A | EL | 22 | 0.539 | 1.47 | A | EL | 2.2 | 0.80 | 0.277 | 1.01 | A | EL | 22 |

44'-0"(BRG. TO BRG.)

1
2
3
END BENT No. 1

END BENT No. 2

LRFR SUMMARY
FOR SPAN 'A'

ASSEMBLED BY: TRP DATE: 11/2014 CHECKED BY: MAL DATE: 11/2014

DRAWN BY: CVC 6/10
CHECKED BY: DNS 6/10

LOAD FACTORS:

	DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
	LOAD RATING	STRENGTH I	1.25	1.5C
FAC	FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

Ι.

3

4.

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.14.R.124

TRANSYLVANIA COUNTY

STATION: 12+45.00 -L-



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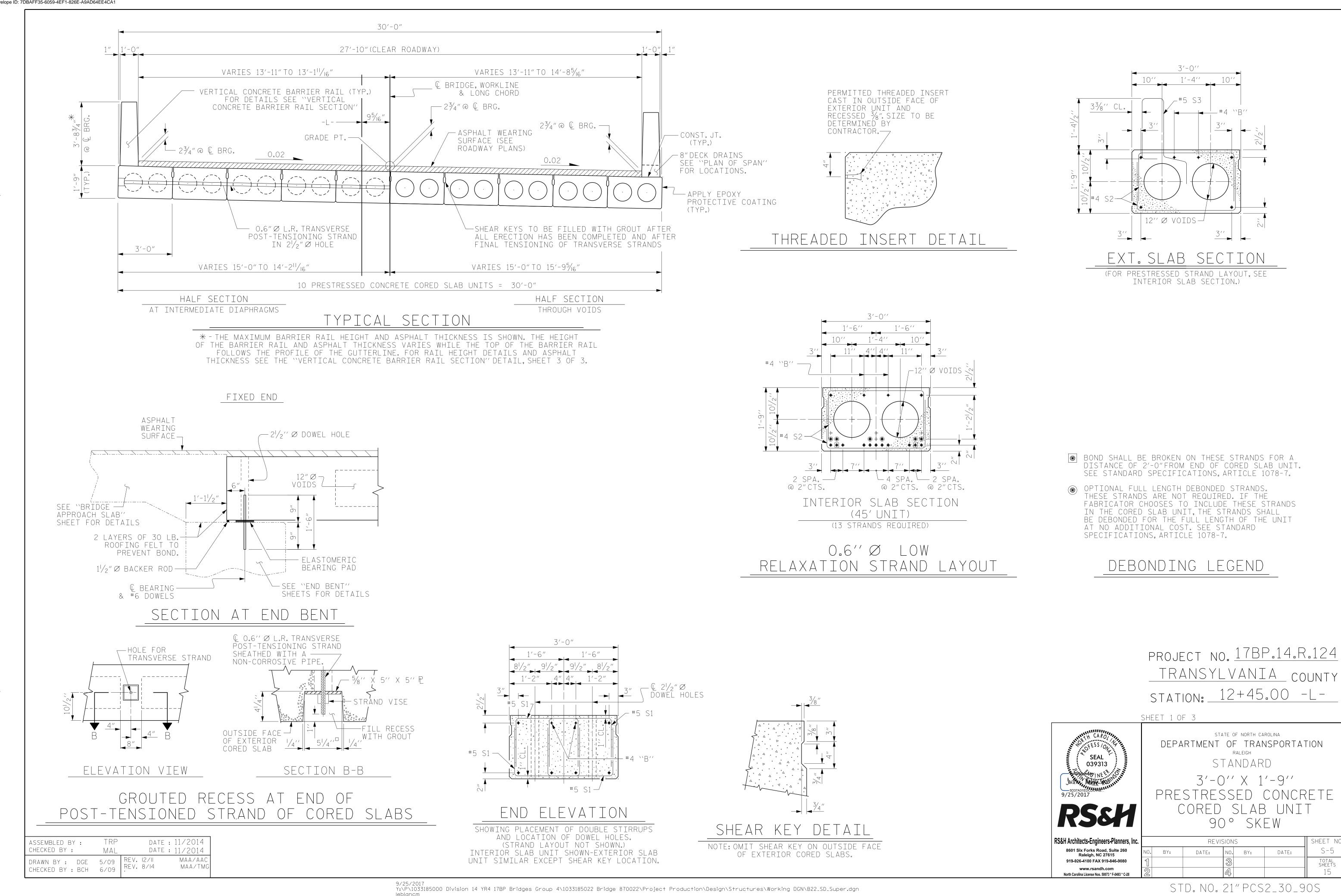
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR

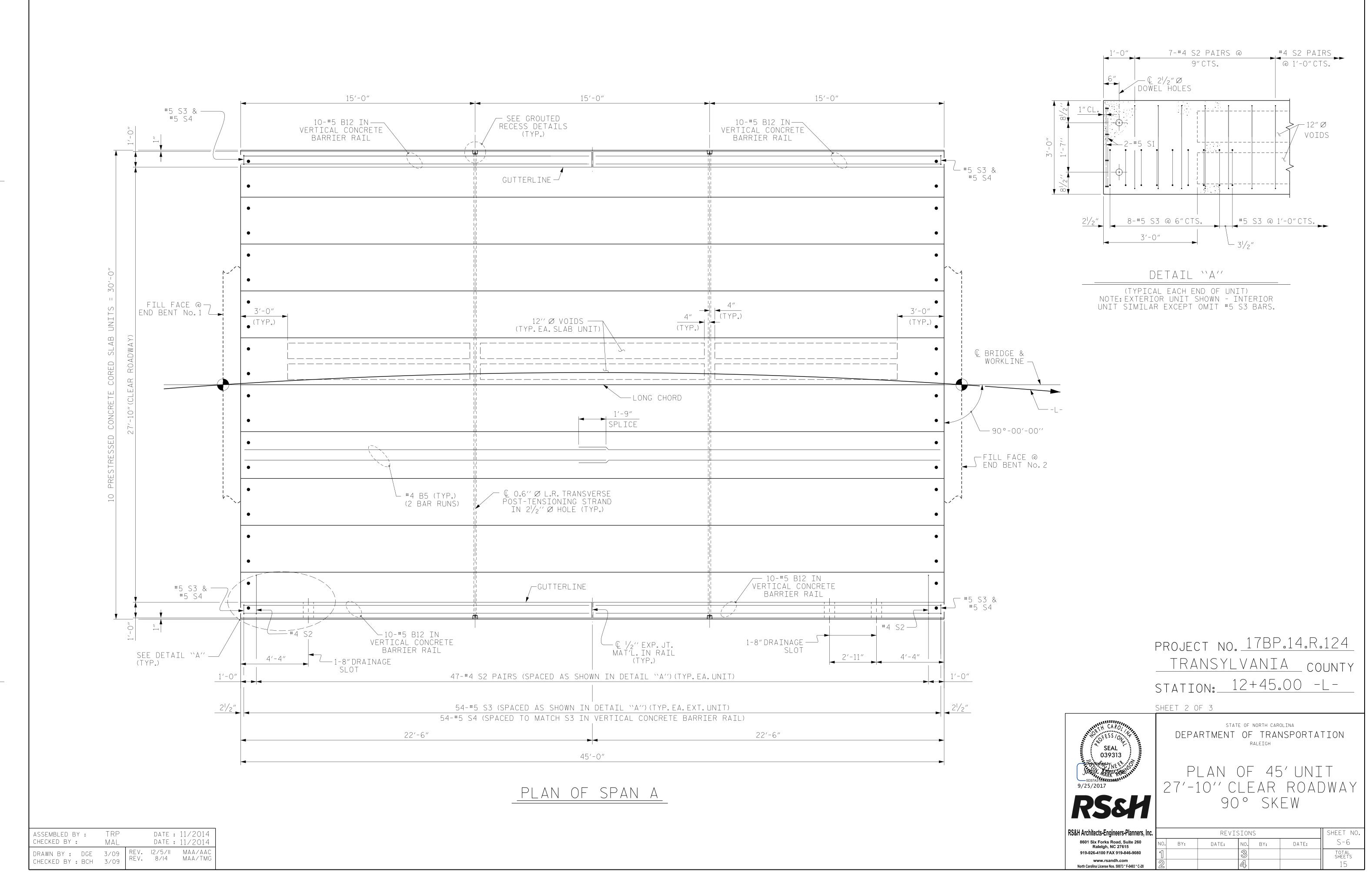
90° SKEW
(NON-INTERSTATE TRAFFIC)

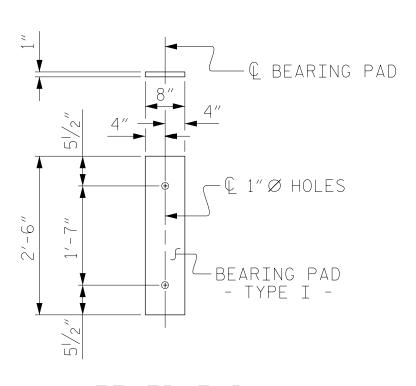
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BY:	DATE:	NO.	BY:	DATE:	S-4
		3			TOTAL SHEETS
		4			15







FIXED END (TYPE I - 20 REQ'D)

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

GUTTERLINE	ASPH	HALT THICKNESS & RAI	L HEIGHT
		ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
		@ MID-SPAN	@ MID-SPAN
45' UNIT		2"	3′-8″

— #5 S4

(TYP.)

23/8" CL.

— #5 S3 m →

- #5 S3 (SEE ``PLAN OF SPAN A" FOR SPACING)

Ç ½″EXP.JT.MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L.

WHEN SLIP FORM IS USED)

CONCRETE BARRIER RAIL SECTION

CHAMFERI

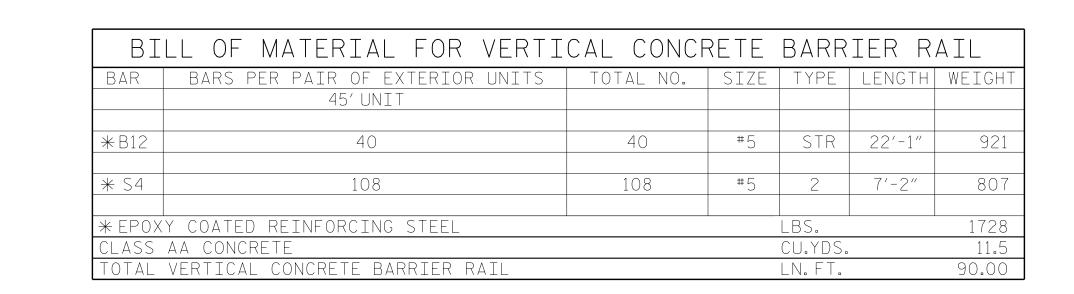
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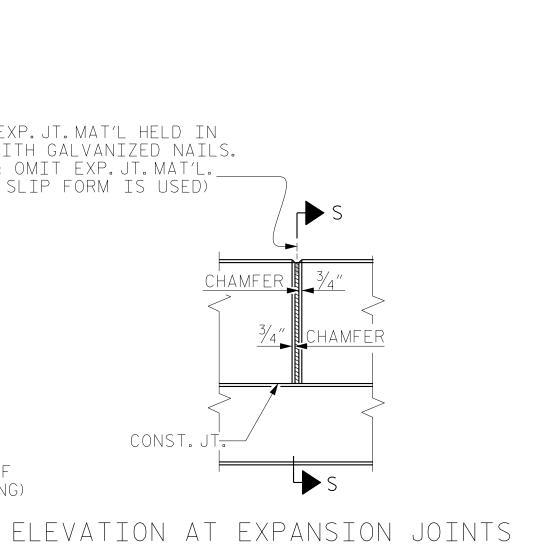
2"CL. MIN.

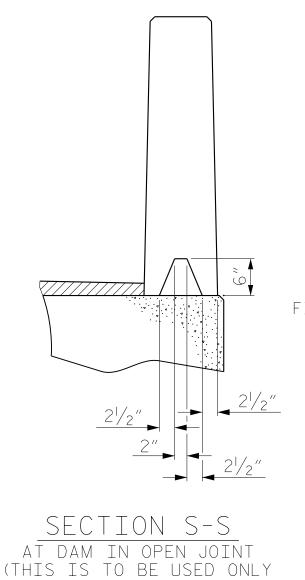
DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
45' CORED SLAB UNIT	0.6″∅ L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	7/8″ ♠
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	V ₈ ″ ♦
FINAL CAMBER	3/4″ ♠

**	INCLUDES	FUTURE	WEARING	SURFACE

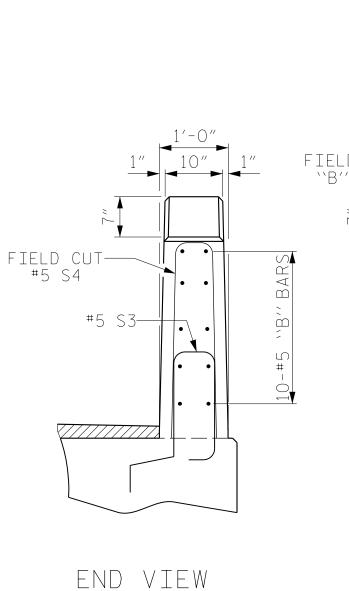
EXTERIOR UNIT INTERIOR UNIT BAR NUMBER SIZE TYPE LENGTH WEIGHT LENGTH WEIGH B5 4 #4 STR 23'-3" 62 23'-3" 62 S1 8 #5 3 4'-3" 35 4'-3" 35 S2 94 #4 3 5'-4" 335 5'-4" 335 **S3 54 #5 1 5'-7" 314 314	BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT										
B5 4 #4 STR 23'-3" 62 23'-3" 62 S1 8 #5 3 4'-3" 35 4'-3" 35 S2 94 #4 3 5'-4" 335 5'-4" 335 ** S3 54 #5 1 5'-7" 314	-										
S1 8 #5 3 4'-3" 35 4'-3" 35 S2 94 #4 3 5'-4" 335 5'-4" 335 ** S3 54 #5 1 5'-7" 314	НТ										
S2 94 #4 3 5'-4" 335 5'-4" 335 ** S3 54 #5 1 5'-7" 314											
S2 94 #4 3 5'-4" 335 5'-4" 335 ** S3 54 #5 1 5'-7" 314											
* S3 54 #5 1 5'-7" 314											
REINFORCING STEEL LBS. 432 432											
* EPOXY COATED											
REINFORCING STEEL LBS. 314											
5000 P.S.I. CONCRETE CU. YDS. 6.5 6.5											
0.6" Ø L.R. STRANDS No. 13											

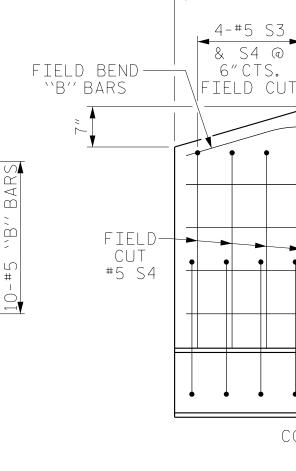






WHEN SLIP FORM IS USED)





BAR TYPES

ALL BAR DIMENSIONS ARE OUT TO OUT

CONCRETE RELEASE STRENGTH

GRADE 270 STRANDS

CORED SLABS REQUIRED

EXTERIOR C.S. 2 45'-0" 90'-0"

(SQUARE INCHES :

JLTIMATE STRENG

(LBS.PER STRAND APPLIED PRESTRES

LBS. PER STRAND

PSI

4000

0.6" Ø L.R.

0.217

58,600

43,950

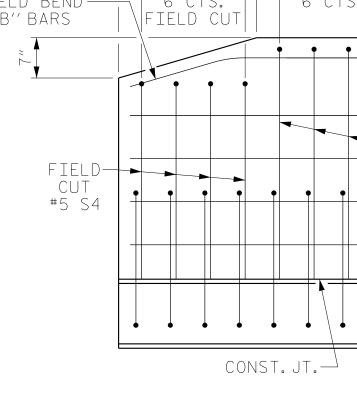
|NUMBER|LENGTH|TOTAL LENGT

UNIT

45' UNIT

45' UNIT

73/4"



SIDE VIEW

4-#5 S3

| & S4 @

#5 S3 & S4

(TYP.)

END OF RAIL DETAILS

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}/_{2}$ " \varnothing dowel holes at fixed ends of slab sections shall be FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS AND EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4"X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

> PROJECT NO. <u>17BP.14.R.124</u> TRANSYLVANIA COUNTY STATION: 12+45.00 -L-SHEET 3 OF 3

SEAL 039313

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 90° SKEW

SHEET NO REVISIONS S-7 DATE: DATE: BY: NO. BY: TOTAL SHEETS

MAL DATE: 11/2014 MAA/AAC REV. 12/II DRAWN BY: DGE 5/09 MAA/TMC CHECKED BY : BCH 6/09

DATE: 11/2014

CONST.JT.—

TRP

ASSEMBLED BY :

CHECKED BY :

TRP MAL

ASSEMBLED BY :

DRAWN BY: MAA 5/10

CHECKED BY: GM 5/10

CHECKED BY :

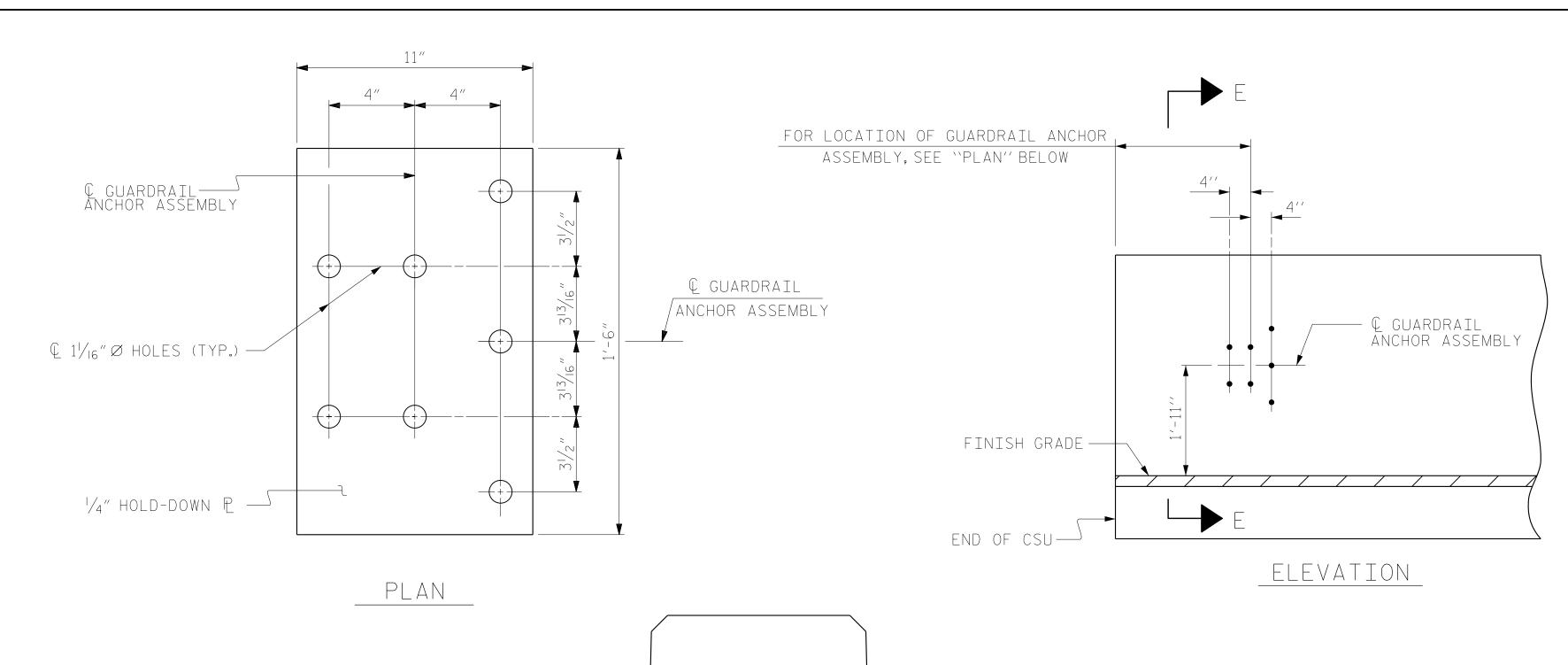
DATE: 11/2014 DATE: 11/2014

MAA/GM

MAA/GM

ADDED 5/6/10

REV. 12/5/II



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4 HOLD DOWN PLATE AND 7 - 1/8 BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

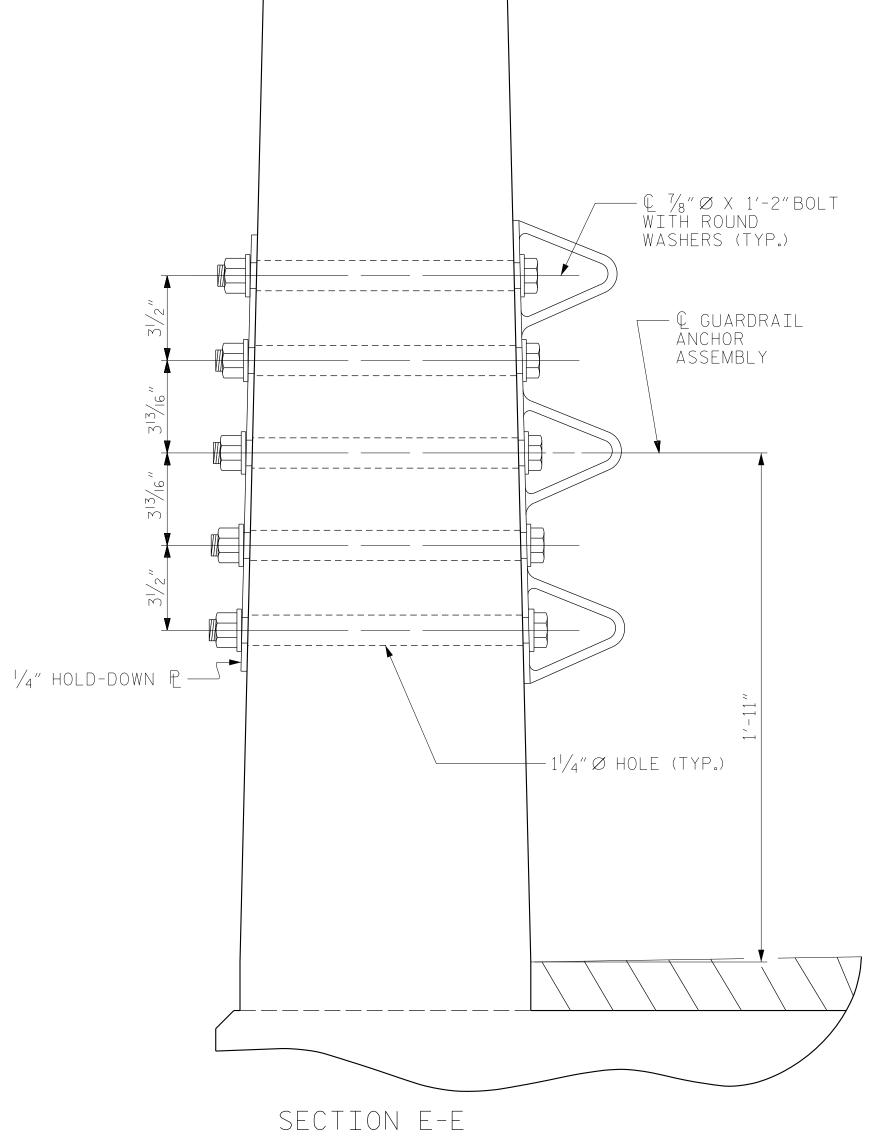
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

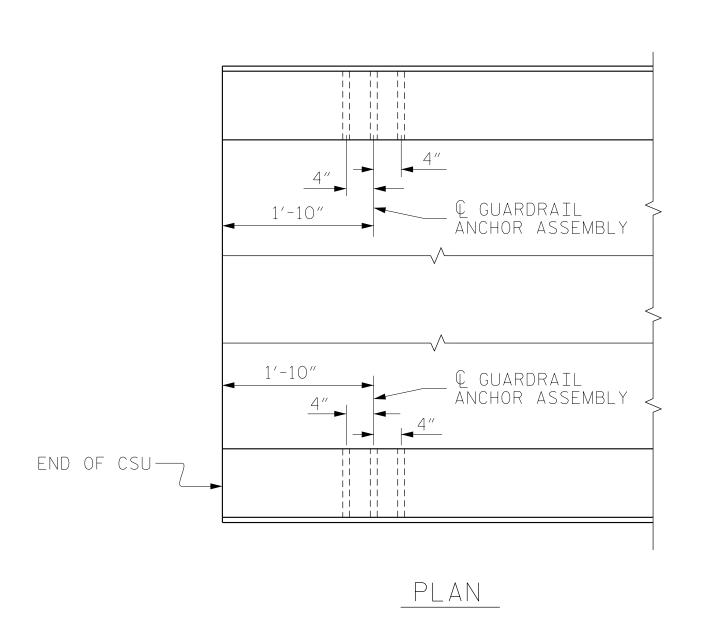
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

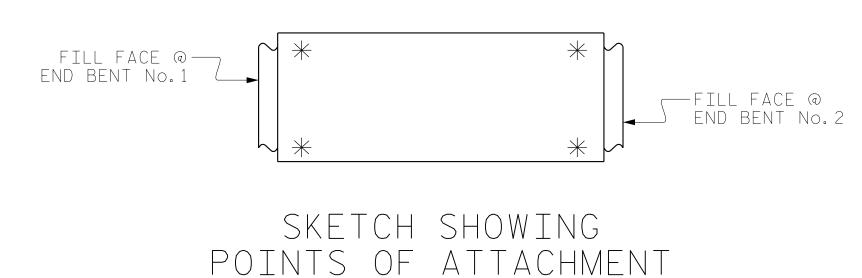
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR.



* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>178P.14.R.124</u> TRANSYLVANIA COUNTY STATION: 12+45.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE BARRIER RAIL

8601 Six Forks Road, Suite 260 Raleigh, NC 27615 919-926-4100 FAX 919-846-9080 www.rsandh.com North Carolina License Nos. 50073 * F-0493 * C-28

RS&H Architects-Engineers-Planners, Inc.

SHEET NO REVISIONS S-8 DATE: BY: DATE: NO. BY: TOTAL SHEETS

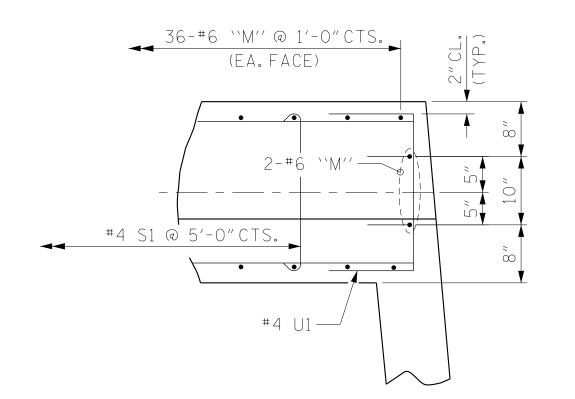
GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

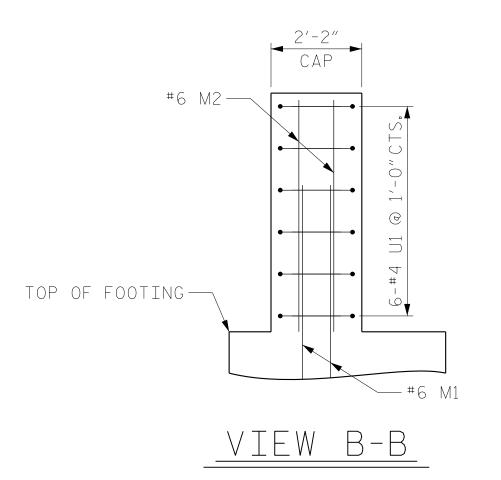
FOR WING DETAILS, SEE SHEET 2 OF 3.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



CAP PART PLAN VIEW

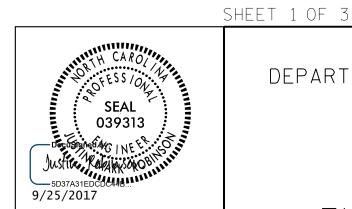
RIGHT END, LEFT END SIMILAR



PROJECT NO. 17BP.14.R.124

TRANSYLVANIA COUNTY

STATION: 12+45.00 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT No. 1

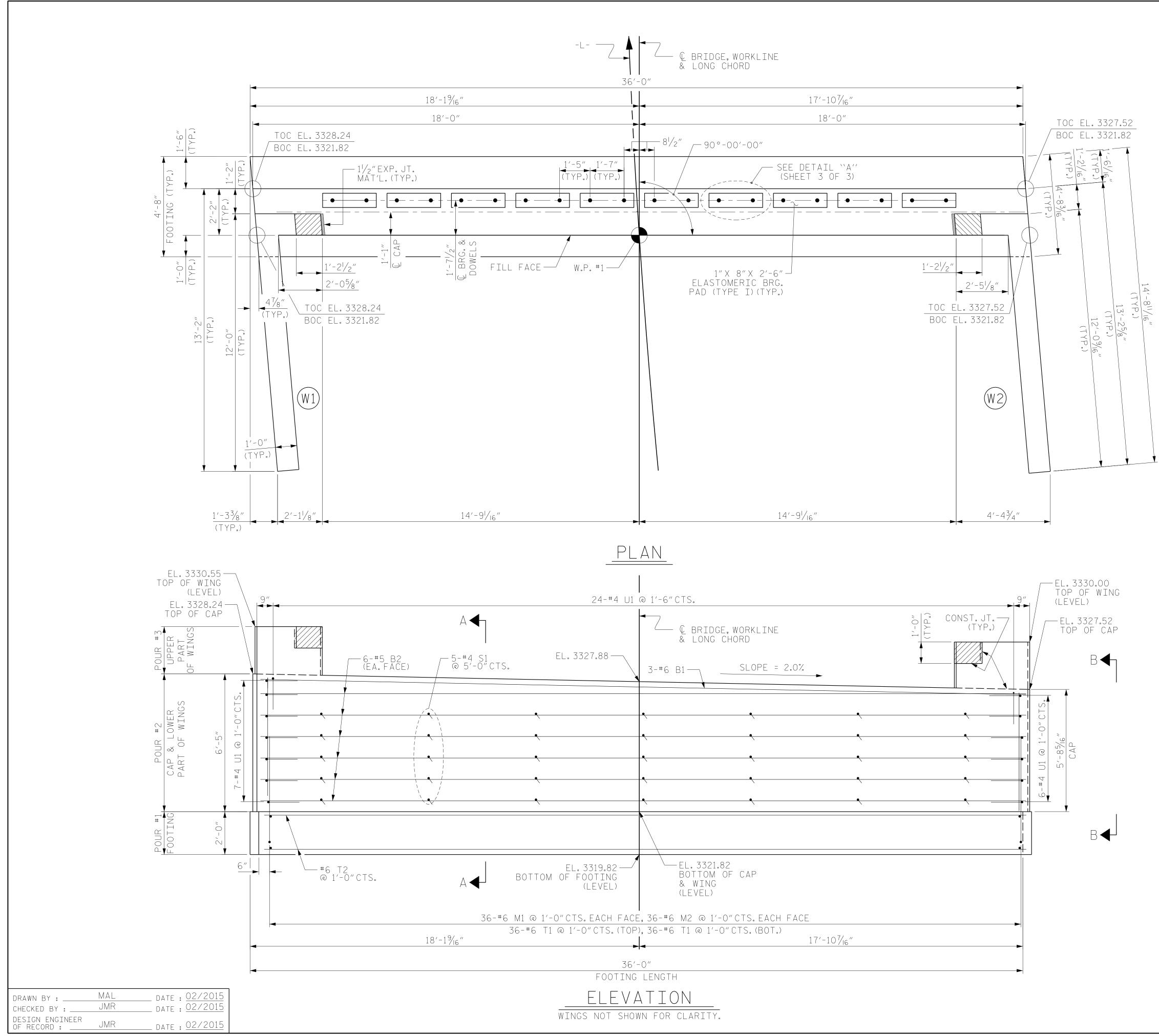
RS&H Architects-Engineers-Planners, Inc.

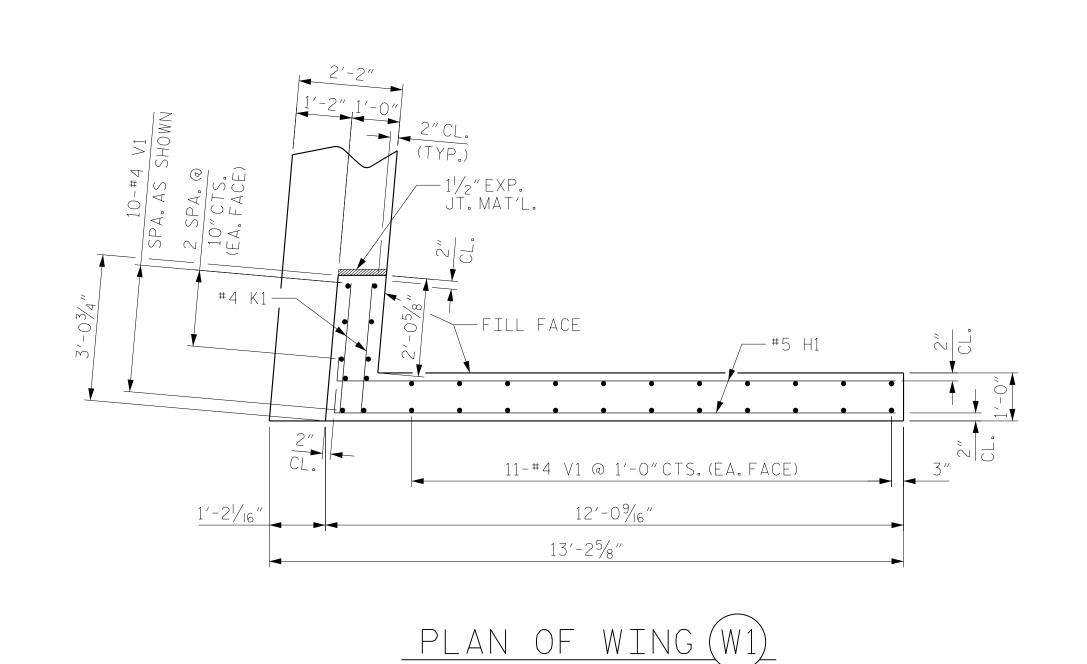
8601 Six Forks Road, Suite 260
Raleigh, NC 27615
919-926-4100 FAX 919-846-9080
www.rsandh.com

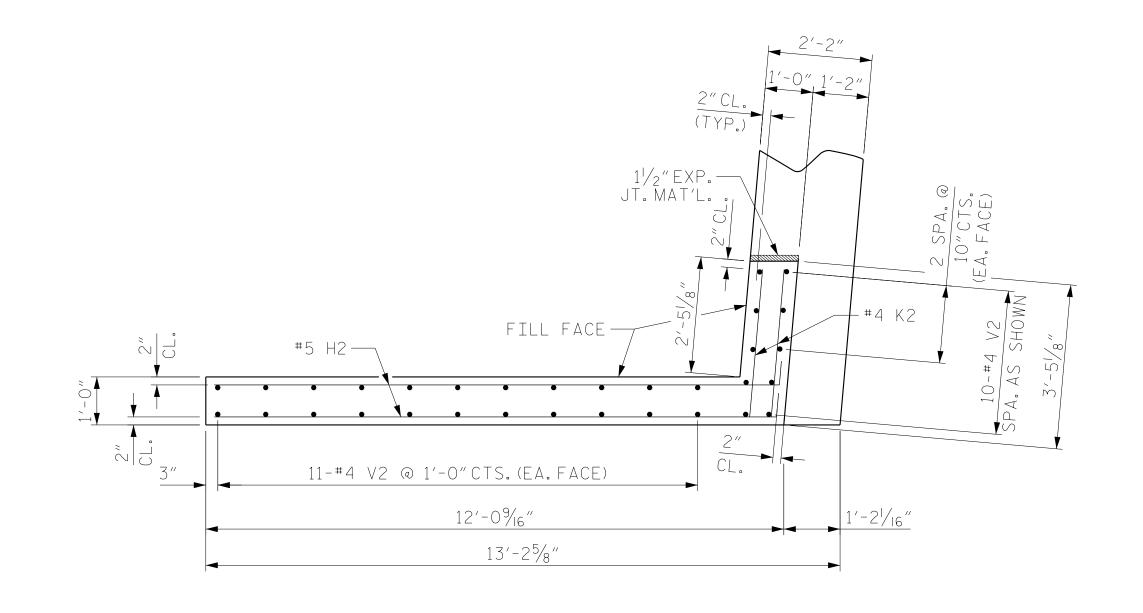
North Carolina License Nos. 50073 * F-0493 * C-28

 ners, Inc.
 REVISIONS
 SHEET NO

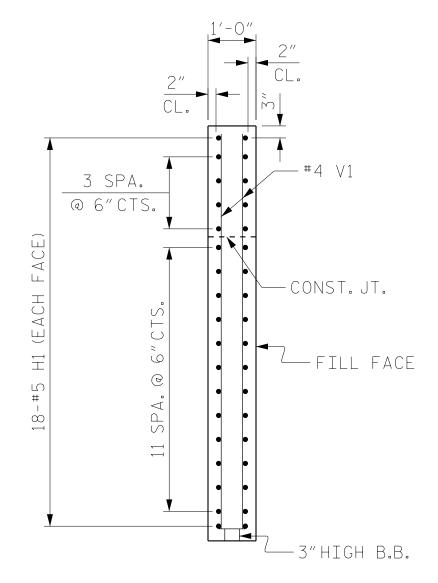
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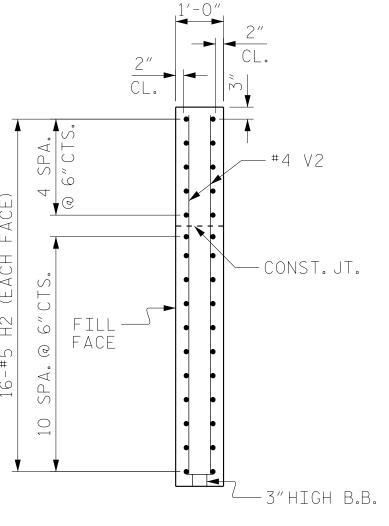




PLAN OF WING (W2)







SECTION Y-Y

PROJECT NO. 17BP.14.R.124

TRANSYLVANIA COUNTY

STATION: 12+45.00 -L-

SHEET 2 OF 3



DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

STATE OF NORTH CAROLINA

END BENT WING DETAILS END BENT No.1

#4 KI (FA. FACE)

SPACED AS SHOWN ABOVE)

TOP OF WING

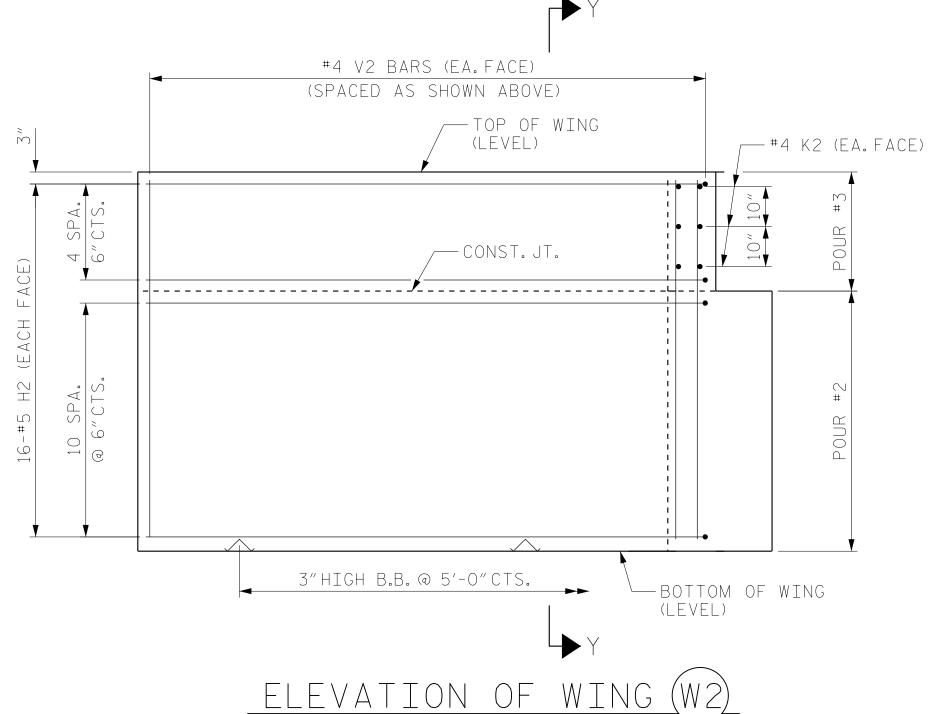
CONST. ..T.

CONST. ..T.

BOTTOM OF WING

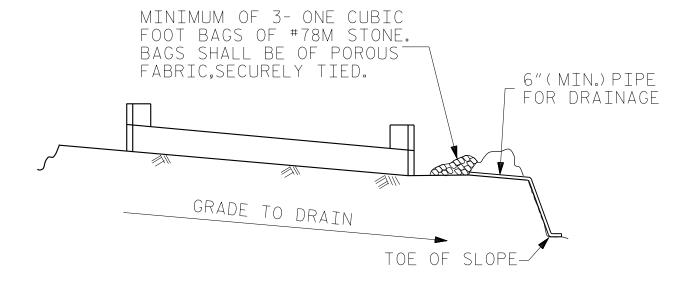
3"HIGH B.B. & 65'-0"CTS.

ELEVATION OF WING (W1)



WING DETAILS

FOOTINGS NOT SHOWN FOR CLARITY

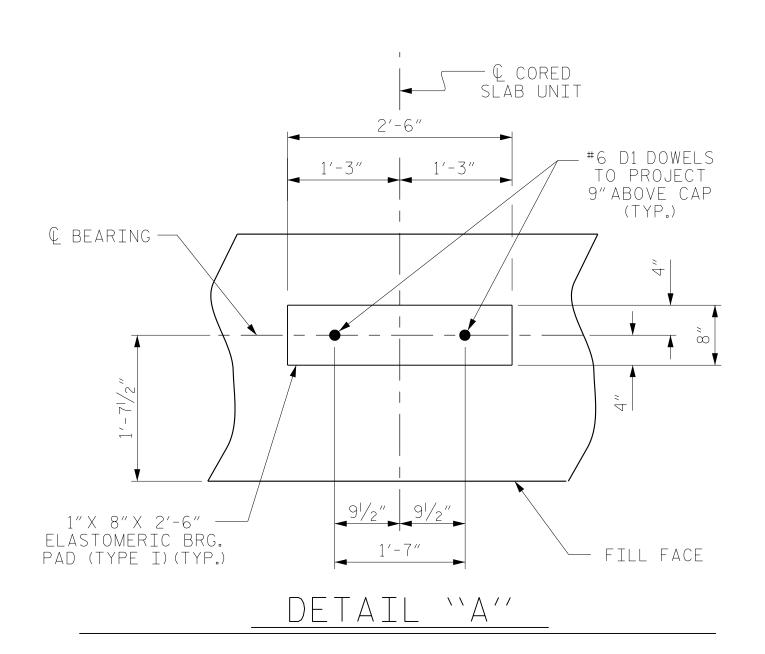


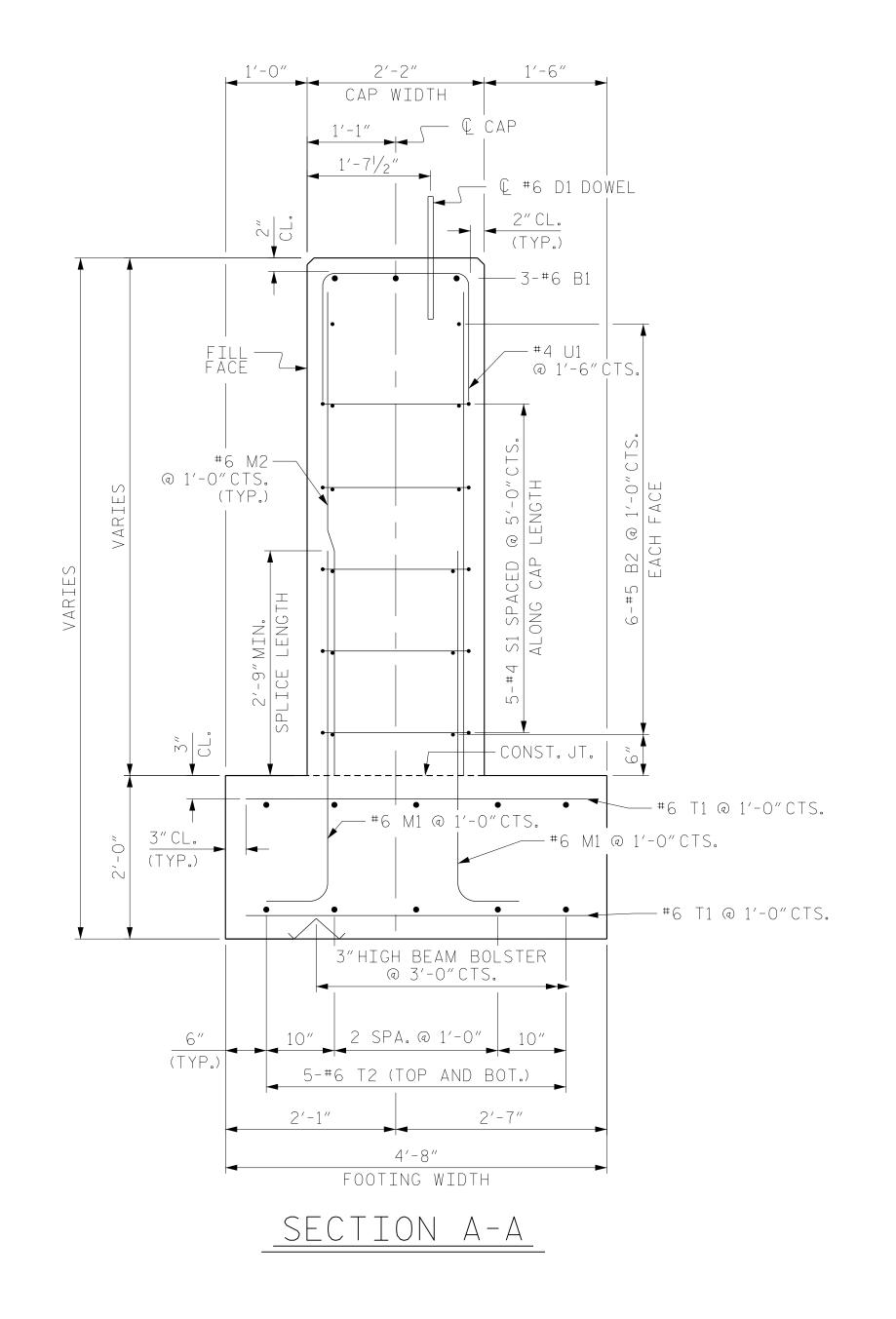
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

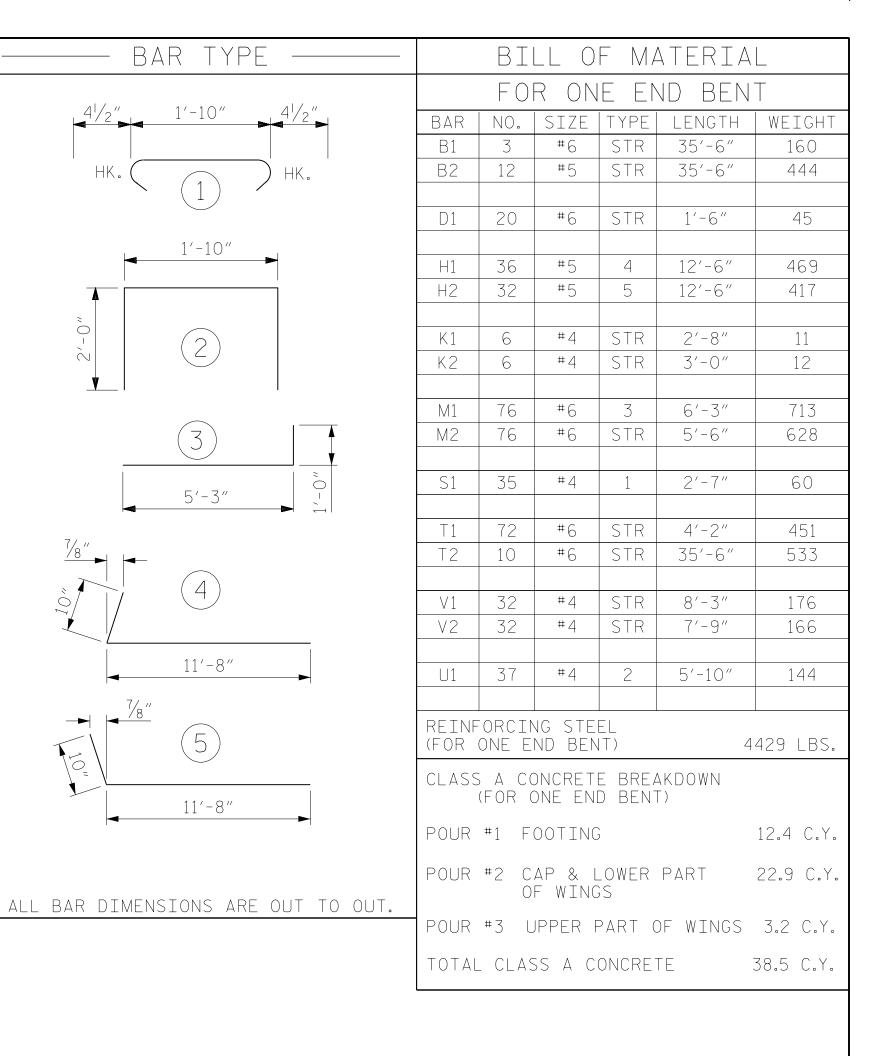
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT







PROJECT NO. 17BP.14.R.124

TRANSYLVANIA COUNTY

STATION: 12+45.00 -L-

SHEET 3 OF 3



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT No. 1 DETAILS

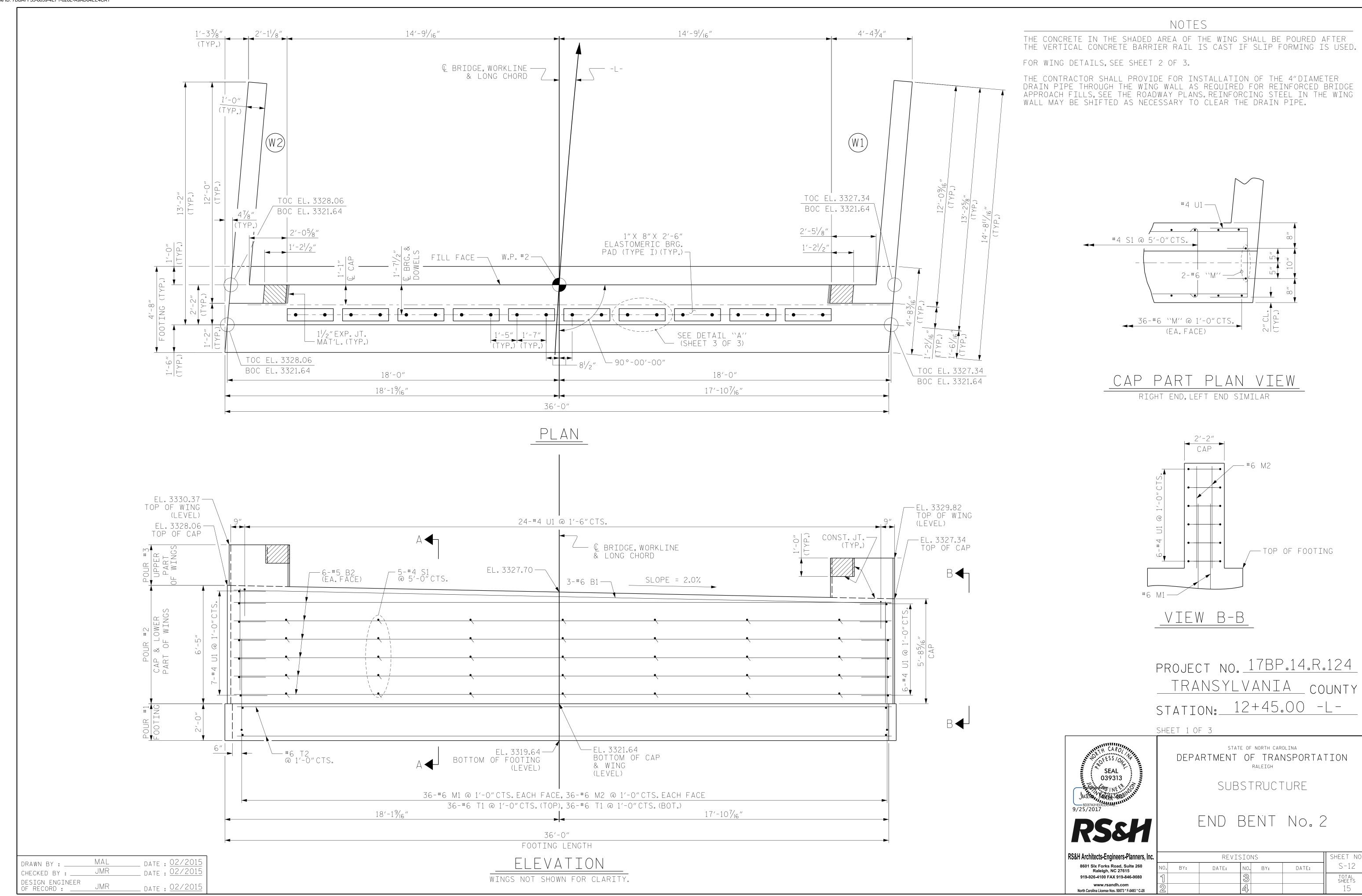
RS&H Architects-Engineers-Planners, Inc.

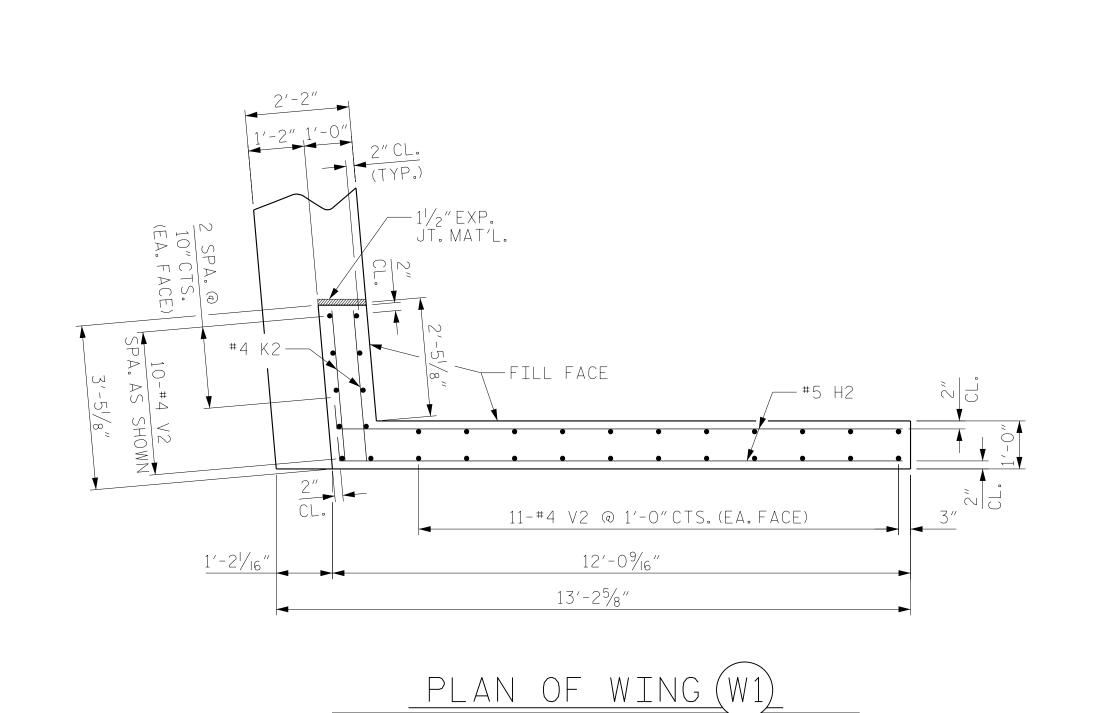
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Raleigh, NC 27615
919-926-4100 FAX 919-846-9080

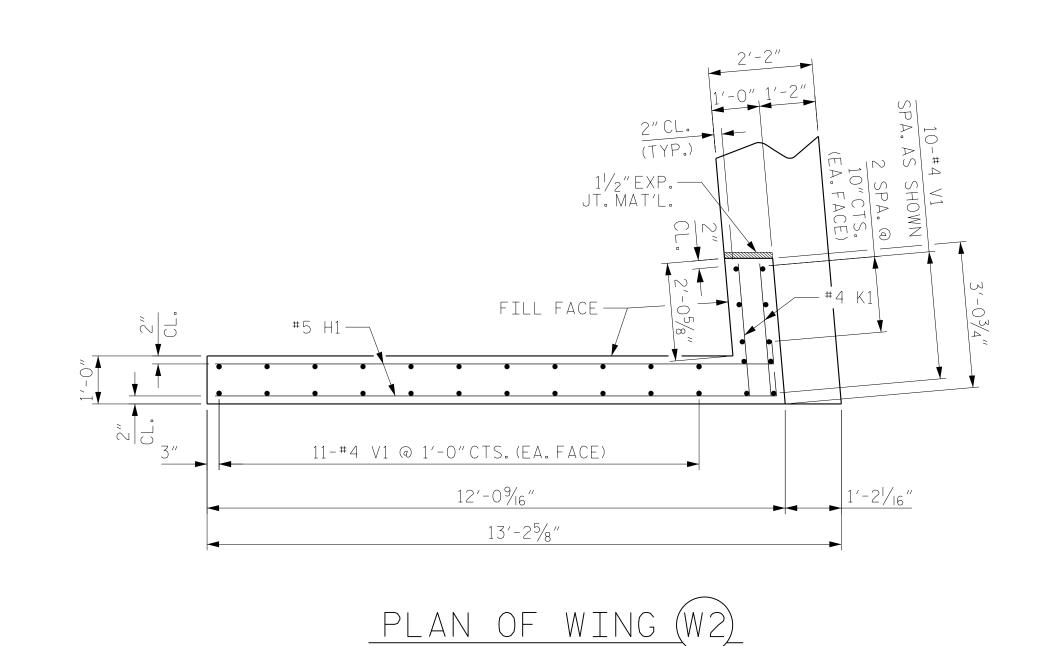
www.rsandh.com

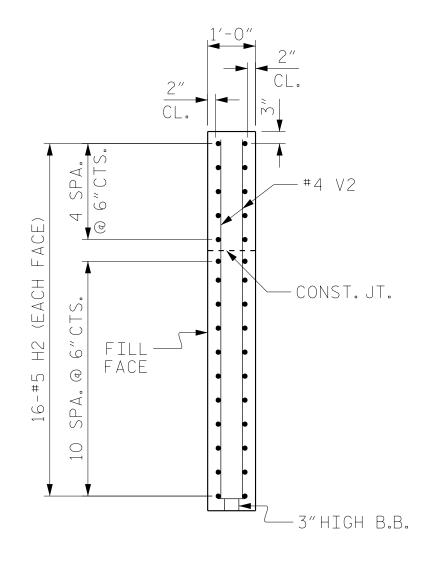
ers-Planners, Inc.			REVIS	SIO	VS		SHEET NO.
d, Suite 260 7615	NO.	BY:	DATE:	NO.	BY:	DATE:	S-11
19-846-9080	1			3			TOTAL SHEETS
com 073 * F-0493 * C-28	2			4			15

DRAWN BY: MAL DATE: 02/2015
CHECKED BY: JMR DATE: 02/2015
DESIGN ENGINEER
OF RECORD: JMR DATE: 02/2015

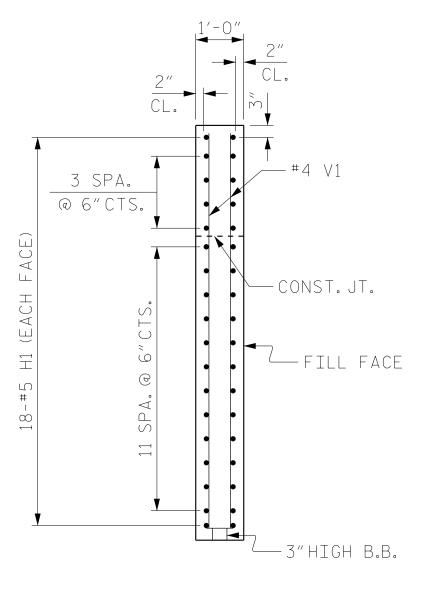








SECTION X-X



<u>SECTION Y-Y</u>

SHEET 2 OF 3

PROJECT NO. <u>17BP.14.R.124</u> TRANSYLVANIA_ COUNTY

STATION: 12+45.00 -L-



SUBSTRUCTURE

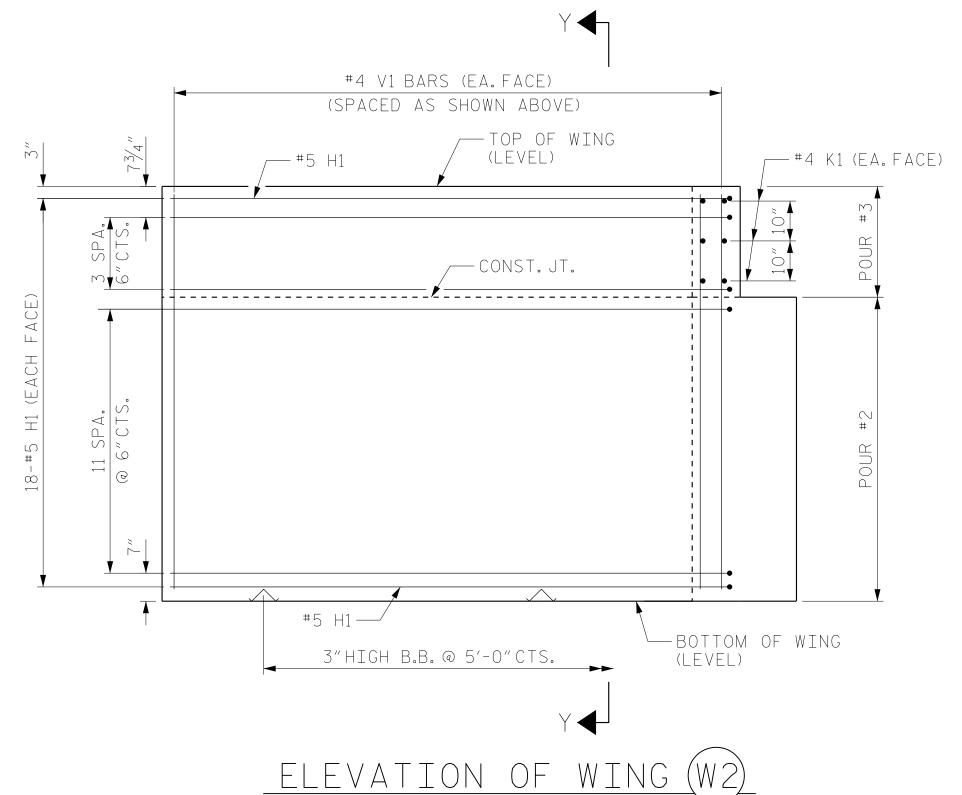
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

END BENT WING DETAILS END BENT No. 2

RS&H Architects-Engineers-Planners, Inc. SHEET NO REVISIONS 8601 Six Forks Road, Suite 260 Raleigh, NC 27615 S-13 DATE: BY: DATE: NO. BY: 919-926-4100 FAX 919-846-9080 TOTAL SHEETS www.rsandh.com North Carolina License Nos. 50073 * F-0493 * C-28

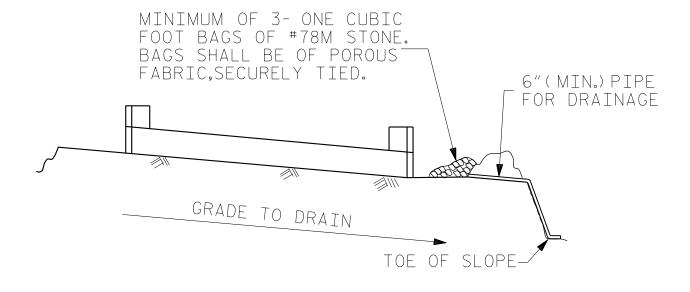
#4 V2 BARS (EA.FACE) (SPACED AS SHOWN ABOVE) TOP OF WING — (LEVEL) #4 K2 (EA.FACE) — CONST.JT.— 3"HIGH B.B. @ 5'-0"CTS. BOTTOM OF WING-(LEVEL) ELEVATION OF WING (W1)



WING DETAILS

FOOTINGS NOT SHOWN FOR CLARITY

__ DATE : <u>02/2015</u> __ DATE : <u>02/2015</u> MAL DRAWN BY : _ JMR CHECKED BY : _ DESIGN ENGINEER OF RECORD : ____ __ DATE : <u>02/2015</u>

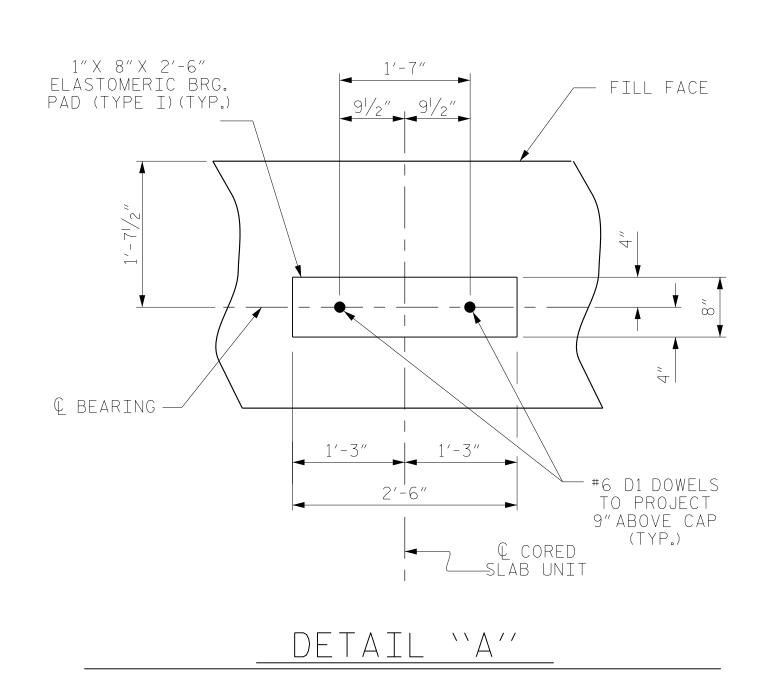


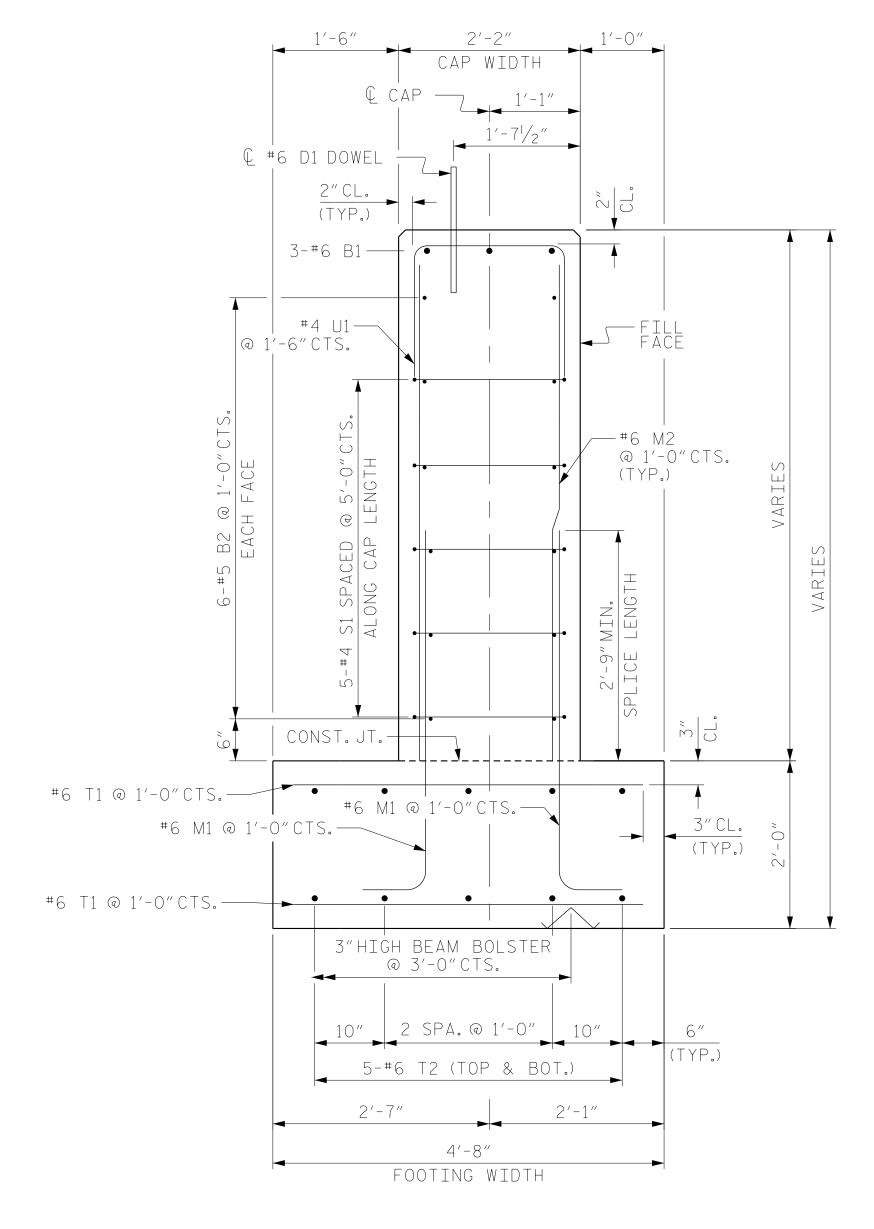
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

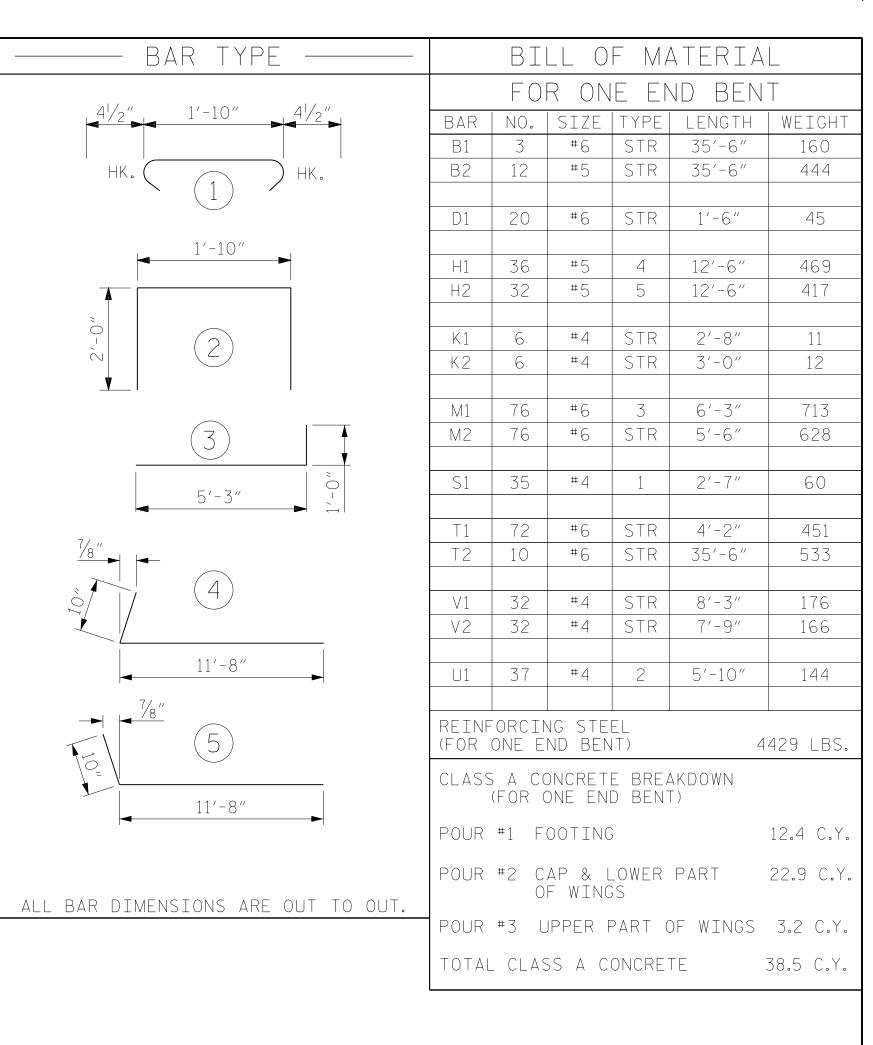
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT





SECTION A-A



PROJECT NO. <u>178P.14.R.124</u> TRANSYLVANIA COUNTY STATION: 12+45.00 -L-

SHEET 3 OF 3



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

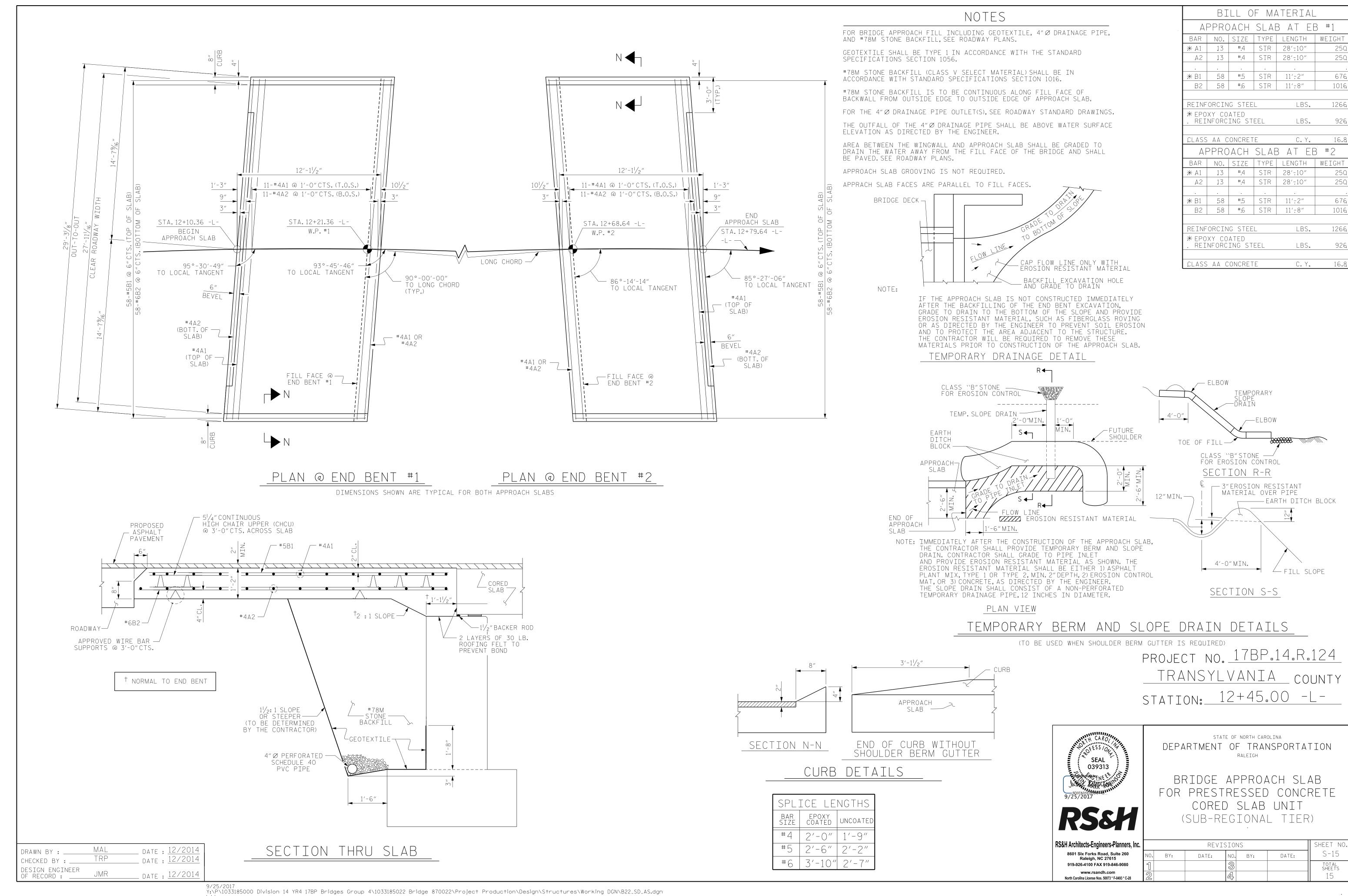
SUBSTRUCTURE

END BENT No. 2 DETAILS

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SHEET NO REVISIONS S-14 DATE: DATE: BY: NO. BY: TOTAL SHEETS North Carolina License Nos. 50073 * F-0493 * C-28

_ DATE : <u>02/2015</u> _ DATE : <u>02/2015</u> DRAWN BY : . JMR CHECKED BY : . DESIGN ENGINEER _ DATE : 02/2015 OF RECORD : ___



STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) ---- SEE PLANS LIVE LOAD IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1.800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER - - - -375 LBS.PER SQ.IN.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH ----

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS.PER CU.FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.

SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH